REVISION SUMMARY REVISION DESCRIPTION

ABBREVIATIONS

A.B.	Anchor Bolt	Flr. Sys.	Floor System	PSF	Pounds per square foot
Abv.	Above		Face Of Masonry	P.T.	Pressure Treated
Adj.	Adjustable	Ft.	Foot / Feet	Rad.	Radius
A.F.F.	Above Finished Floor	Ftg.	Footing	Reg'd.	Required
ALT.	Alternate	Galv.	Galvanized	Rm.	Room
Bm.	Beam	G.C.	General Contractor	Rnd.	Round
B/Beam	Bottom of Beam	G.F.I.	Ground Fault Interrupter	S.F.	Square Ft.
Brg.	Bearing	G.T.	Girder Truss	SHT	Sheet
Cant.	Cantilever	Hdr.	Header	S.L.	Side Lights
Cir.	Circle	Hgt.	Height	S.P.F.	Spruce Pine Fir
Clg.	Ceiling	Int.	Interior	Sq.	Square
CĴ	Control Joint	K/Wall	Kneewall	S.Y.P.	Southern Yellow Pine
Col.	Column	L.F.	Linear Ft.	Thik'n.	Thicken
Cont.	Continuous	Mas.	Masonry	T.O.B.	Top of Block
Dbl.	Double	Max	Maximum	T.O.M.	Top of Masonry
Dia.	Diameter	Min	Minimum	T.O.P.	Top of Plate
Ea.	Each	M.L.	Microlam	Trans.	Transom Window
E.W.	Each Way	Mir.	Mirror	Тур.	Typical
Elec.	Electrical	Mono	Monolithic		Unless Noted Otherwise
Elev.	Elevation	N.T.S.	Not to Scale	Vert.	Vertical
E.O.R	Engineering or Record	O.C.	On center	V.L.	Versalam
Ext.	Exterior	Opn'g.	Opening	VTR	Vent through Roof
Exp.	Expansion	Opt.	Optional	W	Washer
F.B.C.	Florida Bldg. Code	Pc.	Piece	W/	With
Fin. Flr.	Finished Floor	P.L.	Parallam	W.A.	Wedge Anchor
Flr.	Floor	PLF	Pounds per linear foot	Wd	Wood
Fdn.	Foundation	Plt. Ht.	Plate Height	WP	Water Proof

PARK SQUARE HORIZONS WEST BABCOCK RANCH -BLDG (LOTS -

STRUCTURAL DESIGN CRITERIA

CODE CRITERIA

- FLORIDA BUILDING CODE 7TH EDITION (2020) RESIDENTIAL
- FLORIDA FIRE PREVENTION CODE 7TH EDITION (2020)

	SHINGLE / METAL ROOF (PSF)	FLAT ROOF (PSF)	TILE ROOF (PSF)	HEAVY ROOF (PSF
TOP CHORD LL TOP CHORD DL	20 10	30 10	20 15	20 25
BOTTOM CHORD LL* BOTTOM CHORD DL	0 10	0 10	0 10	0 10
TOTAL (PSF)	40	50	45	55
BOTTOM CHORD LL (OPT) ATTICS W/ LIMITED STORAGE ATTICS W/ HEAVY STORAGE * ATTICS W/ NO STORAGE (NON-CONCURRENT)	20 50 10			

GENERAL ROOF LOADING

NOTE: LL REDUCTIONS ARE ALLOWED PER CODE BUT ONLY WITH WRITTEN APPROVAL FROM EOR OR INDICATED ON PLAN

GENERAL FLOOR LOADING

40 (PSF) 10 (PSF) **BOTTOM CHORD LI**

SPECIAL FLOOR LOADING

BALCONIES/ DECKS SALCONIES OVER 100 SQ:FT APPLIED IN ANY DIRECTION AT AN' JARDRAIL IN-FILL COMPONENTS 50 (LBS)(f)

LOAD OF 50 POUNDS ON AN AREA 150(PSF)

TERMITE SPECIFICATIONS

SECTION R318 PROTECTION AGAINST TERMITES

ERMITE PROTECTION SHALL BE PROVIDED BY REGISTERED TERMITICIDES, INCLUDING SOIL APPLIED PESTICIDES, BAITING SYSTEMS, AND PESTICIDES APPLIED TO WOOD, OR OTHER APPROVED METHODS OF TERMITE PROTECTION LABELED FOR USE A PREVENTIVE TREATMENT TO NEW CONSTRUCTION (SEE SECTION 202, REGISTERED TERMITICIDE). UPON COMPLETION OF THE APPLICATION OF THE TERMITE PROTECTIVE TREATMENT. A CERTIFICATE OF COMPLIANCE SHALL BE ISSUED TO THE BUILDING DEPARTMENT BY THE LICENSED PEST CONTROL COMPANY THAT CONTAINS THE FOLLOWING STATEMENT: "THE BUILDING HAS RECEIVED A COMPLETE TREATMENT FOR THE PREVENTION OF SUBTERRANEAN TERMITES. TREATMENT IS IN ACCORDANCE WITH RULES AND LAWS ESTABLISHED BY THE FLORIDA DEPARTMENT OF AGRICULTURE AND CONSUMER SERVICES."

- METHOD OF TREATMENT SHALL BE APPROVED BY THE GOVERNING JURISDICTION "LIQUID BORATE OR BOR-A-COR" PRODUCT METHODS MUST BE DETERMINED AT PERMIT STAGE AND PRODUCT APPROVAL DATA MUST BE ON FILE WITH THE BUILDING DEPARTMENT PRESSURE TREATED LUMBER THAT HAS BEEN CUT OR DRILLED THAT EXPOSES UNTREATED
- PORTIONS OF WOOD ARE REQUIRED TO BE FIELD TREATED TO PREVENT INSECT INFESTATION. OPTIONAL BORATE APPLIED TO ALL FRAME MEMBERS WITHIN 24" A.F.F.

-NOTICE TO BUILDER AND ALL SUBCONTRACTORS-

IT IS THE INTENT OF THE ENGINEER LISTED IN THE TITLEBLOCK OF THESE DOCUMENTS THAT THESE DOCUMENTS BE ACCURATE, PROVIDING LICENSED PROFESSIONALS CLEAR INFORMATION. EVERY ATTEMPT HAS BEEN MADE TO PREVENT ERROR. THE BUILDER AND

- ALL SUBCONTRACTORS ARE REQUIRED TO: REVIEW ALL THE INFORMATION CONTAINED IN THESE DOCUMENTS, PRIOR TO THE COMMENCEMENT OF ANY WORK. THE ENGINEER ARE NOT RESPONSIBLE FOR ANY PLAN ERRORS. OMISSIONS, OR MISINTERPRETATIONS UNDETECTED AND NOT REPORTED TO
- THE ENGINEER PRIOR TO CONSTRUCTION. SHALL STRICTLY OBSERVE ALL APPLICATION CODES DURING THE COURSE OF CONSTRUCTION INCLUDING ALL STATE, CITY, AND COUNTY BUILDING, ZONING, ELECTRICAL, MECHANICAL, PLUMBING AND FIRE CODES. CONTRACTOR SHALL VERIFY
- ALL CODE REQUIREMENTS PRIOR TO COMMENCEMENT OF WORK. THE ARCHITECT / ENGINEER SHALL NOT BE RESPONSIBLE FOR SAFETY PROCEDURES. THE MEANS AND METHODS OF CONSTRUCTION, TECHNOLOGIES, OR THE CONTRACTION TO CARRY OUT THE WORK IN ACCORDANCE WITH THE DRAWINGS AND SPECIFICATIONS
- THE FRAMING PLAN SHOWN INDICATES THE "TRUSS SYSTEM" AND IS THE RESPONSIBILITY OF THE TRUSS SYSTEM ENGINEER (DESIGN PROFESSIONAL OF RECORD). THE TRUSS DESIGN ENGINEER (DELEGATED ENGINEER) HAS FINAL. RESPONSIBILITY FOR EACH INDIVIDUAL TRUSS AND TRUSS PROFILE, AND IS TO SUBMIT A FINAL SET OF TRUSS ENGINEERING SIGNED AND SEALED TRUSS DRAWINGS TO DESIGN PROFESSIONAL OF RECORD FOR REVIEW PRIOR TO FABRICATION
- ANY DISCREPANCY OR ERROR IN DIMENSIONS OR NOTES WITH IN THIS PLAN SHALL BE BROUGHT TO THE ATTENTION OF THE DESIGN PROFESSIONAL FOR CLARIFICATION PRIOR TO CONSTRUCTION.
- ALL CONSTRUCTION MUST BE IN ACCORDANCE TO THE INFORMATION FOUND IN THESE DOCUMENTS. ANY QUESTIONS REGARDING THE INFORMATION FOUND IN THESE PLANS SHOULD BE DIRECTED TO OUR QUALITY ASSURANCE MANAGER AT 321-972-0491 IMMEDIATELY. NO BACK CHARGES WILL BE CONSIDERED FOR REIMBURSEMENT BY THE THE ENGINEER WITHOUT ADVANCED NOTIFICATION AND APPROVAL BY THE ENGINEER. PAYMENTS WILL BE MADE IN ACCORDANCE TO THE TERMS OF THE AGREEMENT.

HOME MAINTENANCE & INSPECTIONS

YEARLY MAINTENANCE AND INSPECTIONS BY THE BUILDER/HOMEOWNER ARE NECESSARY FOR THE FUTURE LIFE OF THIS HOME. CARE MUST BE TAKEN TO CHECK WINDOWS AND DOORS FOR CAULKING, REMOVE LEAVES AND DEBRIS OFF ROOFS, MAKE SURE THAT WATER FLOW IS AWAY FROM THE HOUSE AND HAVE YOUR HOME REPAINTED EVERY 3 - 5 YEARS TO PROTECT THE COATINGS. THE DESIGNER AND ENGINEER OF RECORD ARE NOT RESPONSIBLE FOR THE UPKEEP OF THE HOME AND WILL NOT BE HELD LIABLE FOR INSTANCES THAT MAY OCCUR OVER THE NORMAL LIFE OF THE HOME WITHOUT PROPER MAINTENANCE.

CAST IN PLACE REINFORCED CONCRETE

- ALL CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 2500 PSI (SLABS) 3000 PSI (COLUMNS AND BEAMS), A SLUMP OF 5" PLUS OR MINUS 1", AND HAVE 2 TO 5% AIR ENTRAINMENT, AND A MAXIMUM WATER/CEMENT RATIO OF 0.63 HOOKS SHALL BE PROVIDED AT DISCONTINUOUS ENDS OF ALL TOP BARS OF BEAMS.
- HORIZONTAL FOOTING BARS SHALL BE BENT 25" AROUND CORNERS OR CORNER BARS WITH A 25" LAP PROVIDED EA WAY. CONCRETE COVER MIN. 3" WHEN EXPOSED TO EARTH OR 1 1/2" TO FORM U.N.O.
- FIBER MESH LENGTH SHALL BE ½" TO 2", DOSAGE AMOUNT SHALL BE FROM 1.0 TO 1.5 LBS PER CUBIC YARD IN ACCORDANCE WITH THE MANUFACTURER'S AND SHALL COMPLY WITH ASTM C1116
- ALL REINFORCING STEEL / STIRRUPS AND TIES SHALL BE NEW DOMESTIC DEFORMED BARS FREE FROM RUST. SCALE & OIL & SHALL MEET ASTM A615/ A615M GRADE 60 U.N.O. REINFORCING FOR FOOTING SHALL BE SUPPORTED ON PRE-CAST CONCRETE PADS. STEEL WIRE OR PLASTIC SUPPORT, TOP REINFORCING SHALL BE POSITIVELY SUPPORTED BY TEMPORARY STRINGERS, DOWELS FOR COLUMNS & FILLED CELLS SHALL BE SECURED IN PLACE BY USING ADDITIONAL CROSS- REINFORCING TIED TO FOOTING REINFORCING. SPLICES IN REINFORCING WHERE PERMITTED SHALL BE AS PER DETAIL MS05/L1.
- HIGH STRENGTH SIMPSON SET EPOXY-TIE WAS USED IN THE DESIGN OF THIS PRODUCT. IF CONTRACTORS WISH TO USE A DIFFERENT EPOXY, THEY MUST FIRST CONTACT THE ENGINEER OF RECORD FOR WRITTEN APPROVAL WHERE PROJECT IS TO BE LOCATED IN KNOWN RADON GAS PREVALENT AREAS, APPENDIX "F" OF THE FLORIDA BUILDING CODE 7TH EDITION (2020)
- RESIDENTIAL IS TO BE IMPLEMENTED. F303.4 CONCRETE STRENGTH IN THESE AREAS ARE TO BE A MINIMUM OF 3000 P.S.I. THEREFORE, ANY AND ALL NOTES ON THESE PLANS THAT INDICATE 2500 P.S.I. SHALL BE REPLACED WITH 3000 P.S.I. FOR THE CONCRETE STRENGTH. CONCRETE SLABS SHALL HAVE 6x6-W1.4xW1.4 W.W.F. OR FIBERMESH OVER 6 MIL VISQUEEN VAPOR BARRIER & TREATED FOR TERMITES

- HOLLOW LOAD BEARING UNITS SHALL BE NORMAL WEIGHT, GRADE N, TYPE 2, CONFORMING TO ASTM C90-014, WITH A MINIMUM NET COMPRESSIVE STRENGTH OF 2000 PSI (fm = 2000 PSI)
- MORTAR SHALL BE TYPE "S", CONFORMING TO ASTM C270-14A.
- COARSE GROUT SHALL CONFORM TO ASTM C476-10 WITH A MAXIMUM AGGREGATE SIZE OF 3/8" AND A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS OF 3000 PSI SLUMP 8" TO 11". CONTINUOUS MASONRY INSPECTIONS ARE REQUIRED DURING CONSTRUCTION GRADE 40 U.N.O. VERTICAL REINFORCEMENT SHALL BE AS NOTED ON THE DRAWINGS WITH THE CELLS FILLED WITH COARSE GROUT. GRADE 40 U.N.O.VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT THE TOP AND BOTTOM AND AT A MAXIMUM SPACING OF 192 DIA OR 10FT
- WHICH EVER IS LESS. REINFORCING SHALL BE PLACED IN THE CENTER OF THE MASONRY CELL WITH MIN 1/2" CLEARANCE TO INSIDE FACE. REINFORCING STEEL SHALL BE LAPPED PER DETAIL MS05/L1, UNLESS OTHERWISE NOTED ON THE DRAWINGS GROUT STOPS SHALL BE PROVIDED BELOW BOND BEAM. PLASTIC SCREEN, METAL LATH STRIP OR CAVITY CAPS MAY BE USED TO PREVENT THE FLOW OF GROUT INTO CELLS BELOW. THE USE OF FELT PAPER AS A STOP IS PROHIBITED.
- TEMPORARY BRACING AND SHORING OF WALL TO PROVIDE STABILITY DURING CONSTRUCTION SHALL BE THE RESPONSIBILITY OF THE
- TYPICAL FILLED CELL REINFORCING SIZE AND SPACING SHALL BE ABOVE AND BELOW ALL WALL OPENINGS
- DO NOT APPLY UNIFORM LOADS TO MASONRY WALLS FOR (3) DAYS AND NO CONCENTRATED LOADS FOR (7) DAYS. PER CODE ACI 318-14
- CONSOLIDATE POURS EXCEEDING 12" IN HEIGHT BY MECHANICAL VIBRATION, AND RECONSOLIDATE BY MECHANICAL VIBRATION AFTER INITIAL WATER LOSS AND SETTLEMENT HAS OCCURRED. GROUT SHALL BE FLUSH WITH TOP OF WALL.

- ALL EXTERIOR WOOD STUDS WALLS, BEARING WALLS, SHEAR WALLS, AND MISC. STRUCTURAL WOOD FRAMING MEMBERS, (I.E. BLOCKING OR GABLE END BRACING) SHALL BE EITHER AS SPECIFIED IN PLAN OR IN DETAILS. IF CONFLICTS OCCUR BETWEEN PLAN AND DETAILS. THE STRONGEST MATERIAL SHALL BE USED. AT A MINIMUM, ALL WOOD STRUCTURAL FRAMING MEMBERS SHALL BE SPF #2. ALL LUMBER SPECIFIED ON DRAWINGS ARE INTENDED FOR DRY USE ONLY (MOISTURE CONTENT 19% OR LESS), U.N.O. ALL WATERPROOFING AND FIRE SAFETY SYSTEMS ARE THE RESPONSIBILITY OF THE CONTRACTOR AND ARE TO BE DESIGNED AND DETAILED BY OTHERS
- ANY WOOD FRAME INTERIOR BEARING WALL STUDS THAT HAVE HOLES IN THE CENTER OF THE STUD UP TO 1" DIA. SHALL HAVE STUD PROTECTION SHIELDS. ALL HOLES OVER 1" IN DIA. FOR PLUMBING LINES, ETC. SHALL BE REPAIRED WITH SIMPSON HSS2 STUD SHOES, TYP., U.N.O. MANY OF THE NEW PRESSURE TREATED WOODS USE CHEMICALS THAT ARE CORROSIVE TO STEEL. IT IS THE CONTRACTOR'S RESPONSIBILITY TO VERIFY THE TYPE OF WOOD TREATMENT AND TO SELECT APPROPRIATE CONNECTORS THAT RESIST CORROSION. FOR EXAMPLE, ACQ-C, ACQ-D, CBA-A OR CA-B REQUIRE HOT-DIPPED GALVANIZED OR STAINLESS STEEL FASTENERS. DOT SODIUM BORATE (SBX) DOES NOT.
- UNTREATED WOOD SHALL NOT BE IN DIRECT CONTACT WITH CONCRETE OR MASONRY. SEAT PLATES SHALL BE PROVIDED AT BEARING LOCATIONS WITHOUT WOODEN TOP PLATES.
- SEE PLAN FOR STUD PACK AND BEAM NAILING PATTERNS ALL ENGINEERED LUMBER TO HAVE THE FOLLOWING MIN VALUES U.N.O.

OR C1787, OR AS OTHERWISE APPROVED (REF. 2020 FBC-R-R703.7.1

ALL EXPOSED WOOD OR WOOD IN CONTACT WITH EARTH OR CONCRETE TO BE PRESSURE TREATED.

- PARALLAM COLUMNS: 1.8E Fb = 2400 PSI MICROLAM (LVL) BEAMS: 2.0E Fb= 2600 PS
- GLULAM BEAMS: SP/SP 24F-V5 LAYUP (1.7E FB=2400 PSI) MIN.
- SEE PLAN NOTE FOR ADDITIONAL ROOF, WALL, SHEAR WALL AND FLOOR SHEATHING REQUIREMENTS ALONG W/ NAILING INFORMATION OTHERWISE: 9.1. ROOF DECK: PLYWOOD C-C/C-D. EXTERIOR OR OSB
- 9.2. FLOOR SHEATHING: T&G A-C GROUP 1 APA RATED (48/24) SHEATHING SHALL FINISH FLUSH TO EXTERIOR WALL FACE. 9.3. WALL SHEATHING: $\frac{7}{16}$ " STRUCTURAL I OSB EXPOSURE 1 OR $\frac{15}{30}$ " RATED OSB EXPOSURE 1 (SPECIFIC GRAVITY. G=0.50. MIN.). A MINIMUM $\frac{7}{15}$ " SPACE
- IS RECOMMENDED BETWEEN PANELS AT EDGE AND END JOINTS TO ALLOW FOR EXPANSION. PER R604.3 SHEATHING SHALL NOT BE USED AS WEATHER RESISTANCE BARRIER UNLESS SPECIFIED. LATH AND LATH ATTACHMENTS SHALL BE OF CORROSION-RESISTANT MATERIALS. EXPANDED METAL OR WOVEN WIRE LATH SHALL BE ATTACHED TO WOOD SHEATHING WITH 1½" LONG, 11 GAGE NAILS HAVING A $\frac{7}{16}$ " HEAD, OR 1 $\frac{1}{2}$ " LONG, 16 GAGE STAPLES, SPACED IN ACCORDANCE WITH ASTM C1062

STRUCTURAL STEEL

GENERAL STRUCTURAL NOTES

- MATERIAL SPECIFICATIONS: WIDE FLANGE SECTIONS: ASTM A992. GRADE 50. Fv=50 KSI TUBE STEEL (HSS): ASTM A500. GRADE B. Fv = 46 KSI PIPE STEE ASTM F3125, TYPE E OR S, Fy = 35 KSI ALL OTHER STRUCTURAL & MISC. STEEL: A36 Fy=36 KSI STRUCTURAL CONNECTIONS: ALL STRUCTURAL BOLTS TO
- STRUCTURAL BOLTS SMALLER THAN 5/8" DIA. TO BE A307 THREADED ROD SHALL CONFORM TO A36 OR A307 ANCHOR BOLTS SHALL CONFORM TO ASTM F1554 ALL BOLTS CAST IN CONCRETE: ASTM A36 OR ASTM A-307 SHOP AND FIELD WELDS: E70XX ELECTRODES STEEL REINFORCEMENT SHOP DRAWINGS TO BE PROVIDED TO ENGINEER OF RECORD BEFORE FABRICATION FOR REVIEW AND APPROVAL
- STRUCTURAL CONNECTIONS: ALL STRUCTURAL BOLTS TO BE A325N U.N.O. ALL A325N BOLTS SHALL BE BROUGHT TO A "SNUG-TIGHT" CONDITION . AS DEFINED IN THE SPECIFICATION. SLIP CRITICAL (SC) BOLTS MUST BE FULLY TENSIONED PER SPECIFICATION STRUCTURAL BOLTS SMALLER THAN 5/8" DI. TO BE A307 THREADED ROD SHALL CONFORM TO A36 OR A307 ANCHOR BOLTS SHALL CONFORM TO ASTM F1554 ALL BOLTS CAST IN CONCRETE: ASTM A36 OR ASTM A-307 SHOP AND FIELD WELDS: E70XX ELECTRODES STEEL REINFORCEMENT SHOP DRAWINGS TO BE PROVIDED TO ENGINEER OF RECOR BEFORE FABRICATION FOR REVIEW AND APPROVAL. WELDED CONNECTIONS: ELECTRODES - E70XX UNO (LOW HYDROGEN). FILLET WELDS SHALL BE 3/1
- SHOP DRAWINGS OF ALL STRUCTURAL STEEL SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW PRIOR TO FABRICATION. SHOP DRAWINGS SHALL INCLUDE COMPLETE DETAILS AND SCHEDULES FOR FABRICATION AND ASSEMBLY OF STRUCTURAL STEEL MEMBERS, PROCEDURES. AND DIAGRAMS INCLUDING DETAILS OF CUTS, CAMBERS, HOLES, PROFILES, SIZES, SPACING, AND LOCATIONS OF STRUCTURAL MEMBERS, CONNECTION ATTACHMENTS, FASTENERS, LOAD, TOLERANCES, AND OTHER PERTINENT DATA. INDICATE WELDS BY STANDARD AWS SYMBOLS AND SHOW SIZE. LENGTHS, AND TYPES OF WELDS. PROVIDE SETTING DRAWINGS, TEMPLATES, AND DIRECTIONS FOR INSTALLATION OF ANCHOR BOLTS AND OTHER
- ANCHORAGE TO BE INSTALLED FOR WORK OF OTHER TRADES. STRUCTURAL STEEL SHALL RECEIVE SHOP COAT OF PRIMER (COLOR AS DIRECTED BY ARCHITECT) EXCEPT FOR AREAS WHICH WILL RECEIVE SPRAY-ON
- A CERTIFIED TESTING AGENCY SHALL BE ENGAGED TO PERFORM INDUSTRY STANDARD INSPECTIONS TO ENSURE CONFORMANCE WITH PLANS AND SPECIFICATIONS (IF PROVIDED). SUBMIT REPORTS TO ARCHITECT AND ENGINEER.

PRE ENGINEERED WOOD TRUSSES

- ALL PREFABRICATED WOOD TRUSSES SHALL BE SECURELY FASTENED TO THEIR SUPPORTING WALLS OR BEAMS WITH HURRICANE CLIPS OR ANCHORS PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED IN ACCORDANCE WITH THE LATEST EDITION OF THE "NATIONAL DESIGN SPECIFICATION FOR
- STRESS-GRADE LUMBER AND ITS FASTENERS" AS RECOMMENDED BY THE NATIONAL FOREST PRODUCTS ASSOCIATION. TRUSS MEMBERS AND CONNECTIONS SHALL BE PROPORTIONED (WITH A MAXIMUM ALLOWABLE STRESS INCREASE FOR LOAD DURATION OF 25%) TO WITHSTAND THE LIVE LOADS GIVEN IN THE NOTES AND TOTAL DEAD LOAD. BRIDGING FOR PRE-ENGINEERED TRUSSES SHALL BE AS REQUIRED BY THE TRUSS MANUFACTURER UNLESS NOTED ON THE PLANS.
- TRUSS ELEVATIONS AND SECTIONS ARE FOR GENERAL CONFIGURATION OF TRUSSES ONLY. WEB MEMBERS ARE NOT SHOWN, BUT SHALL BE DESIGNED BY THE TRUSS MANUFACTURER IN ACCORDANCE WITH THE FRAMING DESIGN LOADS: DESIGN SPECIFICATIONS FOR LIGHT WEIGHT METAL PLATE CONNECTED WOOD TRUSSES PER THE TRUSS PLATE INSTITUTE TPI LATEST EDITION.
- PRE-ENGINEERED WOOD TRUSSES SHALL BE DESIGNED BY THE MANUFACTURER IN ACCORDANCE WITH SPECIFIED LOADS AND GOVERNING CODES SUBMITTALS SHALL INCLUDE TRUSS FRAMING PLANS AND DETAILS SHOWING MEMBER SIZES, BRACING, ANCHORAGE, CONNECTIONS, TRUSS LOCATION AND PERMANENT BRACING AND/OR BRIDGING AS REQUIRED FOR ERECTION AND FOR THE PERMANENT STRUCTURE. EACH SUBMITTAL SHALL BE SIGNED AND SEALED BY A FLORIDA REGISTERED STRUCTURAL ENGINEER. SUBMIT 3 COPIES FOR REVIEW AND APPROVAL PRIOR TO FABRICATION. THE TRUSS MANUFACTURER SHALL DETERMINE ALL SPANS WORKING POINTS, BEARING POINTS, AND SIMILAR CONDITIONS. TRUSS SHOP DRAWINGS

SHALL SHOW ALL TRUSSES, ALL BRACING MEMBERS, AND ALL TRUSS TO TRUSS HANGERS.

UPLIFT CONNECTORS SUCH AS HURRICANE CLIPS, TRUSS ANCHORS AND ANCHOR BOLTS ARE ONLY REQUIRED ON MEMBERS IN WALLS THAT ARE EXPOSED TO UPLIFT OR LATERAL FORCES. INTERIOR LOAD BEARING WALLS ARE NOT ALWAYS EXPOSED TO UPLIFT FORCES. THE MEMBERS OF THESE WALLS WOULD NOT NEED TO HAVE CONNECTORS APPLIED. PLEASE COORDINATE THE TRUSS ENGINEER FOR THE LOCATION OF THESE WALLS.AND STRUCTURAL PLANS FOR MORE INFO.

- MISSED "J" BOLTS FOR WOOD BEARING WALLS MAY BE SUBSTITUTED WITH 1/2" DIA. EPOXY ANCHORS WITH 7" EMBEDMENT. SIMPSON "SET" EPOXY ADHESIVE BINDER FOLLOWING ALL MANUFACTURER'S RECOMMENDATIONS OR SIMPSON 1/2" TITEN HD BOLTS WITH MINIMUM 7" EMBEDMENT. SEE PLAN SOLD NOTES & SCHEDULES FOR EMBEDMENT DEPTH AT FLOOR STEPS.
- FOR MISSED VERT. DOWELS, DRILL A 3/4" DIAMETER HOLE 6" DEEP AT THE LOCATION OF THE OMITTED REBAR AND INSTALL A 32" LONG #5 BAR INTO THE EPOXY FILLED HOLE. USE A TWO PART EMBEDMENT EPOXY (SIMPSON HIGH STRENGTH EPOXY-TIE ANCHORING ADHESIVE) MIXED PER THE MANUFACTURER'S INSTRUCTIONS. ASSURE THAT ALL DUST AND DEBRIS FROM DRILLING ARE REMOVED FROM THE HOLE BY BRUSHING AND USING COMPRESSED AIR PRIOR TO APPLYING THE EPOXY. ALLOW THE EPOXY TO CURE TO THE MANUFACTURER'S SPECIFICATIONS, THEN FILL THE CELL IN THE
- MISSED LINTEL STRAPS FOR MASONRY CONSTRUCTION MAY BE SUBSTITUTED WITH (1) SIMPSON MTSM16 TWIST STRAP W/ (4) 1/4"x 21/4" TITENS TO MASONRY AND (7)-10d NAILS TO TRUSS FOR UPLIFTS LESS THAN 860 LBS (USE (2) MTSM16 FOR UPLIFTS LESS THAN 1660#). IF CORNER STRAP IS MISSED, CONTRACTOR IS TO INSTALL (2) SIMPSON HGAM10 W/ (4) 1/4" x 1 1/2" SDS SCREWS AND (5) 1/4" x 2 1/4" TITENS ONE EACH SIDE OF TRUSS.
- NO MORE THAN 10 STRAPS MAY BE SUBSTITUTED OR NO MORE THAN 3 IN A ROW WITHOUT APPROVAL FROM EOR. IF GIRDER TRUSS CONNECTIONS ARE MISSED. CONTACT THE EOR FOR SUBSTITUTION. IF MISSED, MSTAM36 OR MSTAM40 STRAP IS MISSED FOR 2ND FLOOR JAMB STUD CONNECTION, CONTRACTOR MAY INSTALL SIMPSON HTT5 W/ (26) 16d x 21/2" NAILS AND 5/8" ANCHOR BOLT SET IN SIMPSON HIGH STRENGTH EPOXY W/ MIN 6" EMBEDMENT AND MIN 3" EDGE DISTANCE. CONTACT EOR IF STRAPS ARE MISSED UNDER GIRDER JAMB STUD LOCATIONS.

FOR MORTAR JOINTS LESS THAN 1/4", PROVIDE (1) #5 VERT. IN CONC. FILLED CELL EACH SIDE OF THE JOINT (BAR DOES NOT HAVE TO BE CONT. TO

WIND LOADING CRITERIA

- WIND SPEED (ALLOWABLE) 116.0 MPH EXPOSURE CÂTEGORY BUILDING CATEGORY
- ENCLOSURE CLASSIFICATION **ENCLOSED** INTERNAL PRESSURE COEFFICIENT NOTE: MEAN ROOF HEIGHT FOR TYPICAL SINGLE STORY HOME IS 15FT, AND FOR 2 STORY HOME IS 30FT

EFFECTIVE WIND PRESSURE AND SUCTION (PSF)

ASCE 7-16 WALL DESIGN ALLOWABLE COMPONENT AND CLADDING WIND PRESSURES AND SUCTIONS FOR MEAN ROOF HEIGHT ≤ 60 ft

WIND AREA (SQ FEET)	١,) VALUE DENO -) VALUE DENC			WIND PRESSURE AND SUCTION DIAGRAM		
AREA		4		(5)	_		
10 - 19.99	A	(+) 34.0 (-) 36.4	B	(+) 34.0 (-) 44.8			
20 - 49.99	©	(+) 32.5 (-) 35.0	D	(+) 32.5 (-) 42.0			
50 - 99.99	E	(+) 30.4 (-) 32.2	E	(+) 30.4 (-) 37.8	5		
> 100	G	(+) 28.8 (-) 30.8	H	(+) 28.8 (-) 35.0	(4) (5) (5)		
GARA	AGE D	OORS*		SOFFIT			
9'-0" x 7'-0"	'	16'-0" x 7'-0"			a a		
(+) 29.8	J	(+) 28.6 (-) 31.8		(+) 34.0	DIAGRAM		

GENERAL PRESSURE NOTES

MULTIPLY THE ABOVE PRESSURES BY 1.67 TO GET ULTIMATE WIND

- PRESSURES "a" = END ZONE IS ONLY WITHIN 4'-0" OF ALL EXTERIOR BUILDING CORNERS. INDICATED PRESSURES CAN BE INTERPOLATED FOR OTHER DOOR SIZES,
- OTHERWISE USE LOAD ASSOCIATED WITH THE LOWER EFFECTIVE AREAS. DESIGNATED AREAS WHERE THE ULTIMATE WIND SPEED IS 140 MPH OR GREATER AND IS CONSIDER TO BE IN THE WIND-BOURNE DEBRIS AREA. CONTRACTOR TO PROVIDED ADDITIONAL INFO AS REQUIRED FOR PERMITTING.

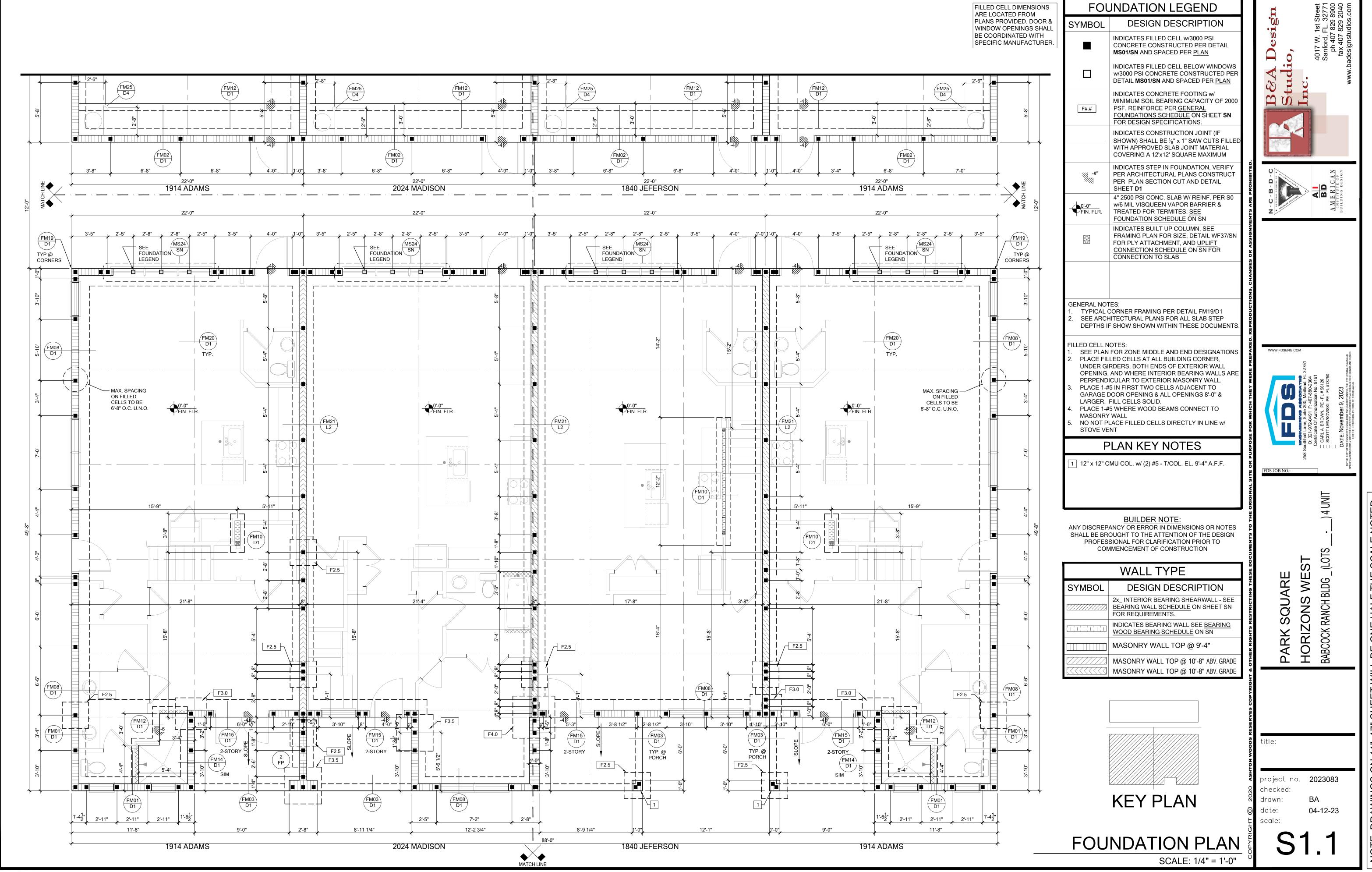
SHEET INDEX

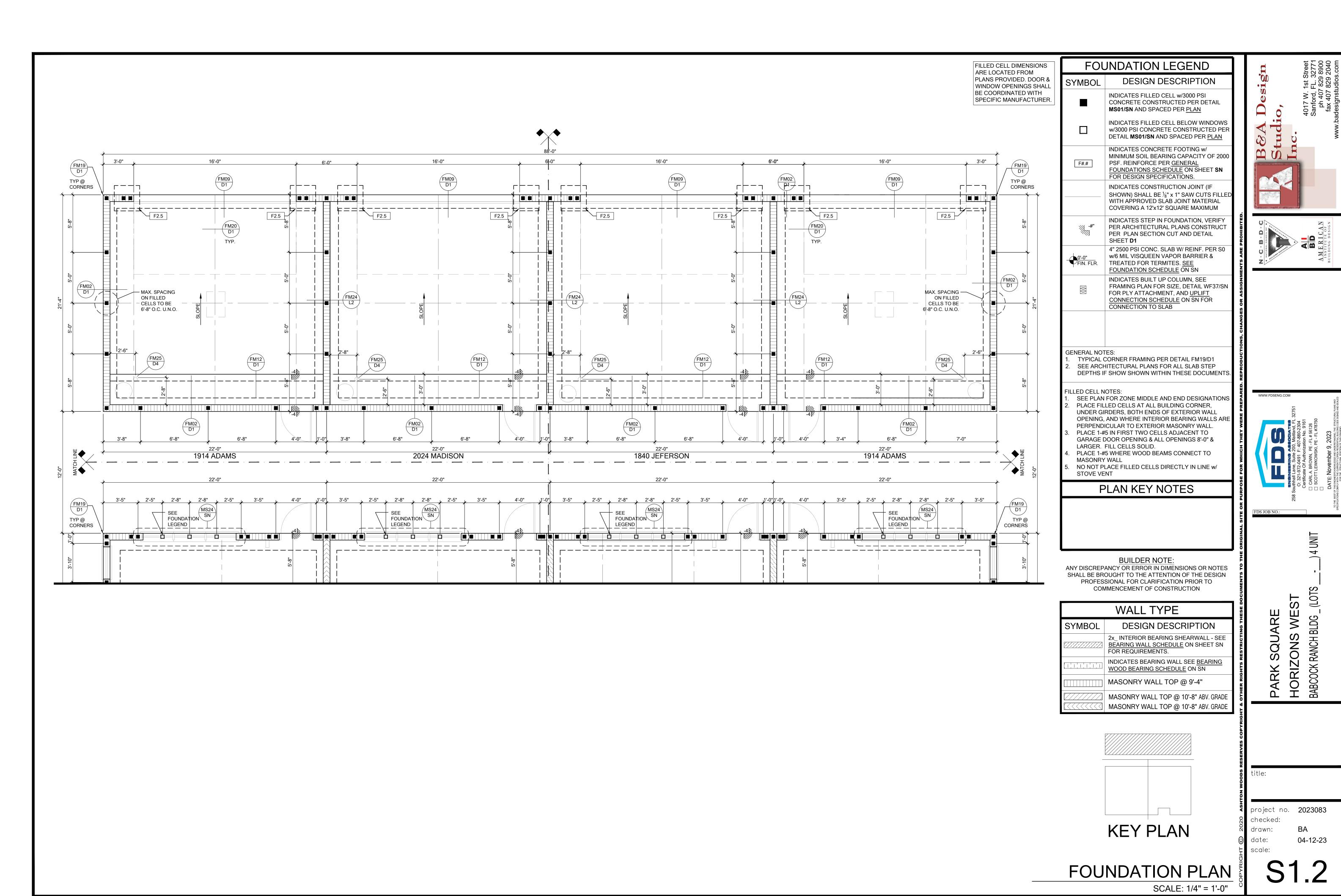
V	S0	NOTES & SCHEDULES	SN	NOTES & SCHEDULES
=	S1.1	FOUNDATION PLAN	D1	FOUND. DETAILS
łΕ	S1.2	FOUNDATION PLAN	D2	FRAMING DETAILS
IL	S2.1	FLOOR FRAMING PLAN	D3	FRAMING DETAILS
	S2.2	FLOOR FRAMING PLAN	D4	FRAMING DETAILS
Ο,	S2.3	FLOOR FRAMING PLAN	FP	FIRE PROTECTION DETAIL
E	S3.1	ROOF FRAMING PLAN		
	S3.2	ROOF FRAMING PLAN		
	L1	LINTEL PLAN		

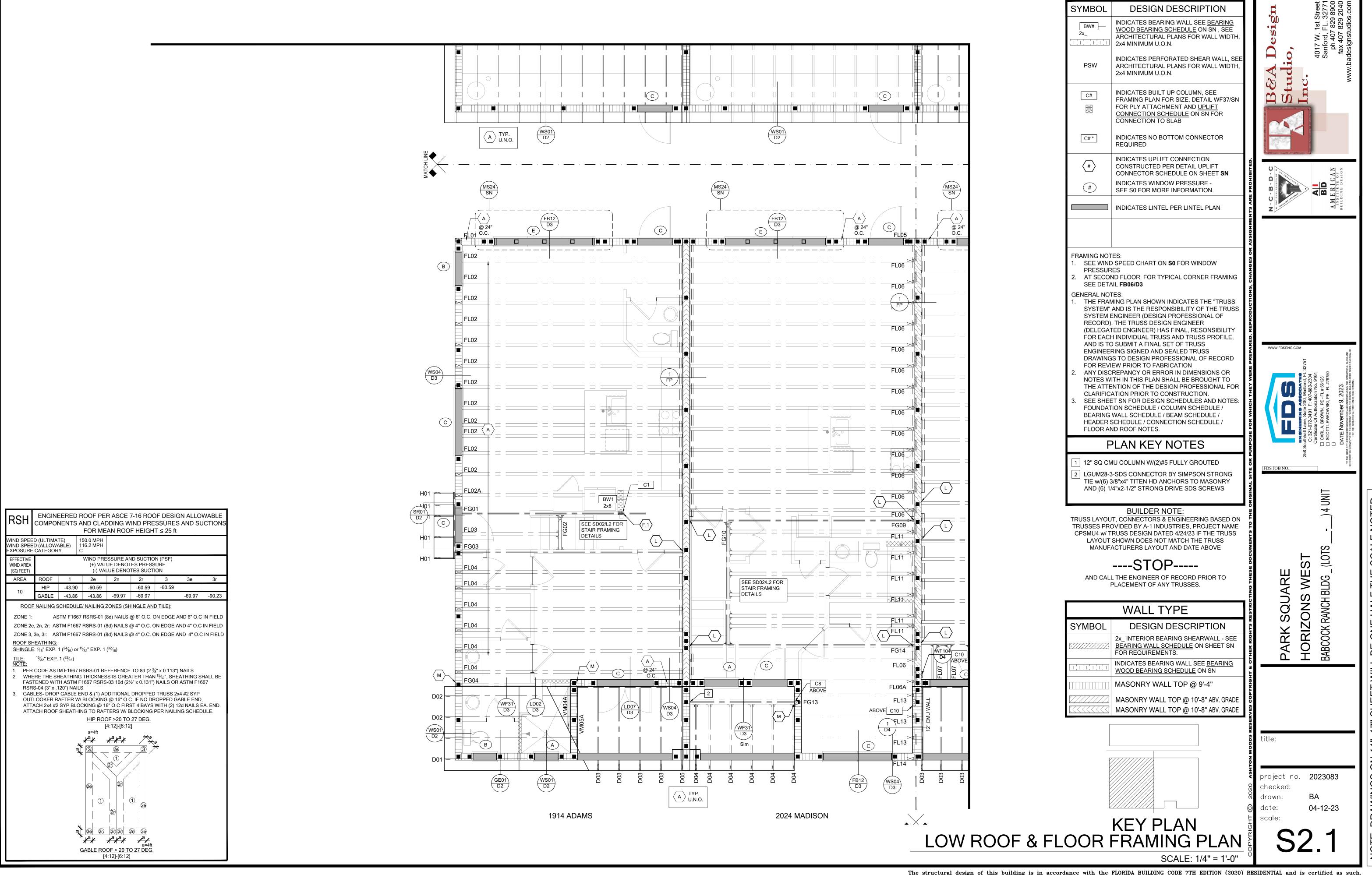
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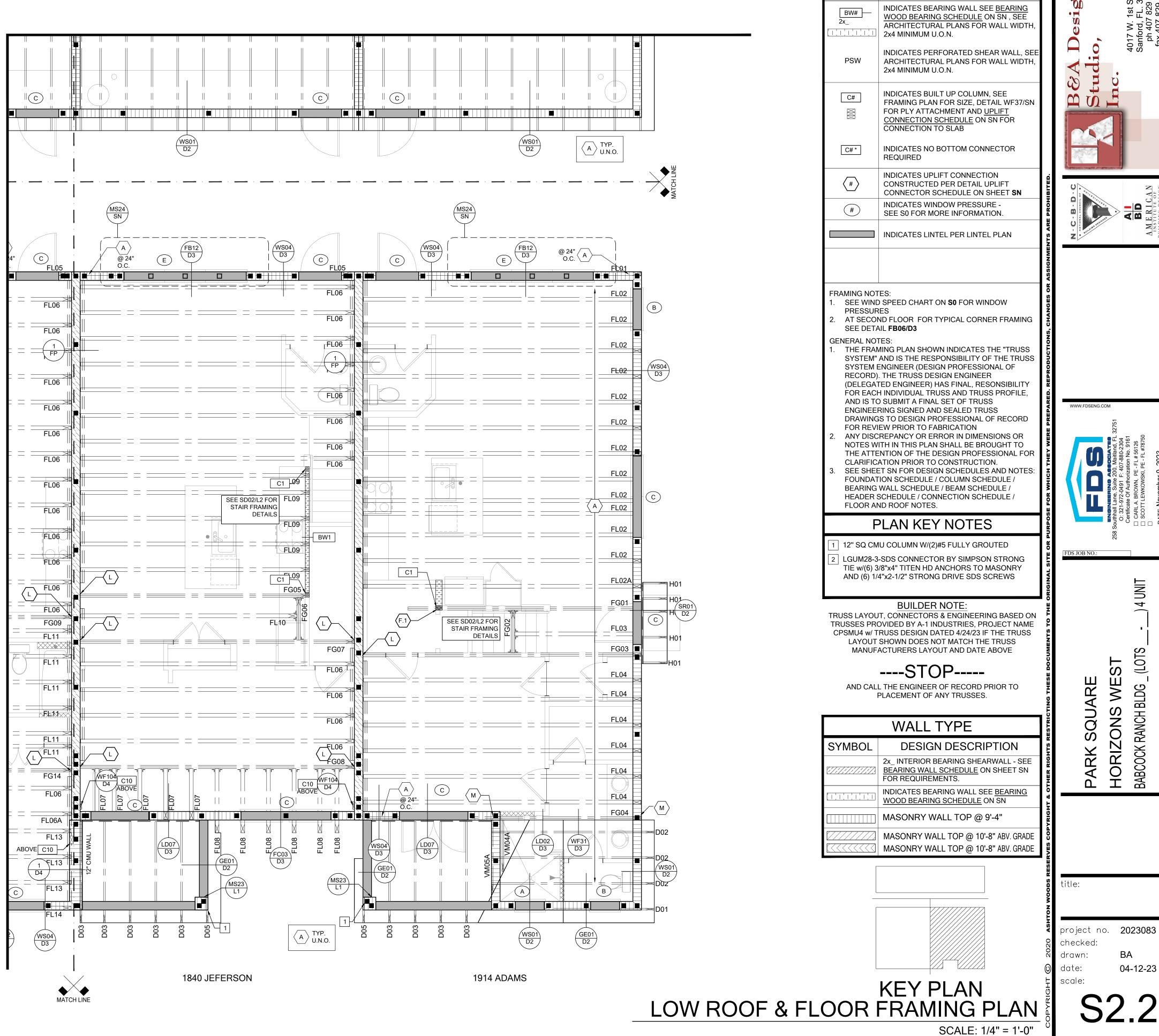
FDS JOB NO.:

L2 LINTEL CHART & NOTES The structural design of this building is in accordance with the FLORIDA BUILDING CODE 7TH EDITION (2020) RESIDENTIAL and is certified as such.









ENGINEERED ROOF PER ASCE 7-16 ROOF DESIGN ALLOWABLE

COMPONENTS AND CLADDING WIND PRESSURES AND SUCTIONS

FOR MEAN ROOF HEIGHT ≤ 25 ft

(+) VALUE DENOTES PRESSURE

(-) VALUE DENOTES SUCTION

ASTM F1667 RSRS-01 (8d) NAILS @ 6" O.C. ON EDGE AND 6" O.C IN FIELD

ZONE 2e, 2n, 2r: ASTM F1667 RSRS-01 (8d) NAILS @ 4" O.C. ON EDGE AND 4" O.C IN FIELD ZONE 3, 3e, 3r: ASTM F1667 RSRS-01 (8d) NAILS @ 4" O.C. ON EDGE AND 4" O.C IN FIELD

WHERE THE SHEATHING THICKNESS IS GREATER THAN 15/32", SHEATHING SHALL BE

FASTENED WITH ASTM F1667 RSRS-03 10d (2½" x 0.131") NAILS OR ASTM F1667

ATTACH ROOF SHEATHING TO RAFTERS W/ BLOCKING PER NAILING SCHEDULE. HIP ROOF >20 TO 27 DEG.

> GABLE ROOF > 20 TO 27 DEG. [4:12]-[6:12]

GABLES- DROP GABLE END & (1) ADDITIONAL DROPPED TRUSS 2x4 #2 SYP

OUTLOOKER RAFTER W/ BLOCKING @ 16" O.C. IF NO DROPPED GABLE END, ATTACH 2x4 #2 SYP BLOCKING @ 16" O.C FIRST 4 BAYS WITH (2) 12d NAILS EA. END.

. PER CODE ASTM F1667 RSRS-01 REFERENCE TO 8d (2 $\frac{3}{8}$ " x 0.113") NAILS

-60.59 -60.59

WIND SPEED (ALLOWABLE) 116.2 MPH

HIP -43.90 -60.59

SHINGLE: $\frac{1}{16}$ " EXP. 1 ($\frac{24}{16}$) or $\frac{15}{32}$ " EXP. 1 ($\frac{32}{16}$)

GABLE -43.86 -43.86 -69.97 -69.97

ROOF NAILING SCHEDULE/ NAILING ZONES (SHINGLE AND TILE):

WIND AREA

(SQ FFFT)

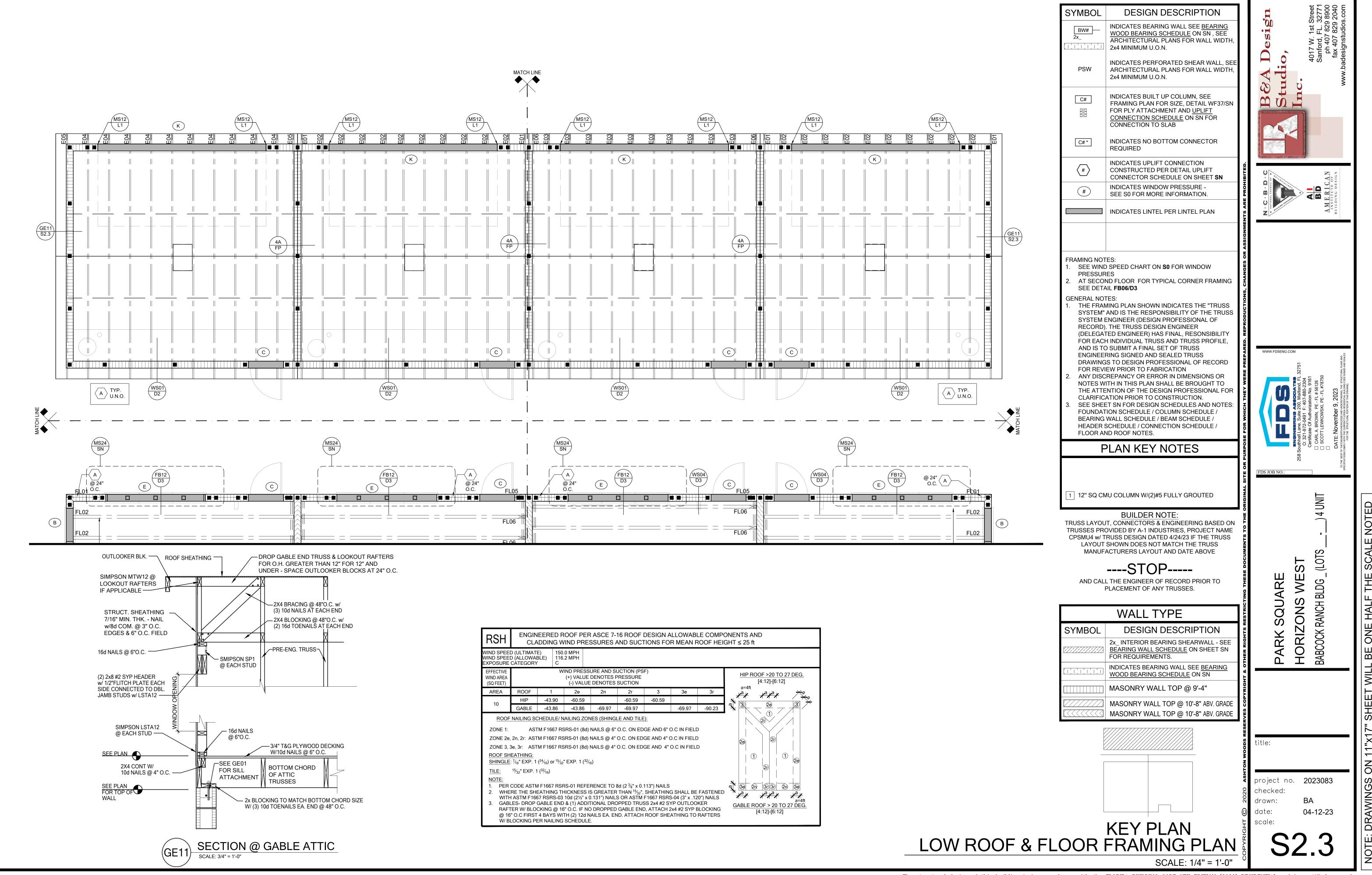
AREA

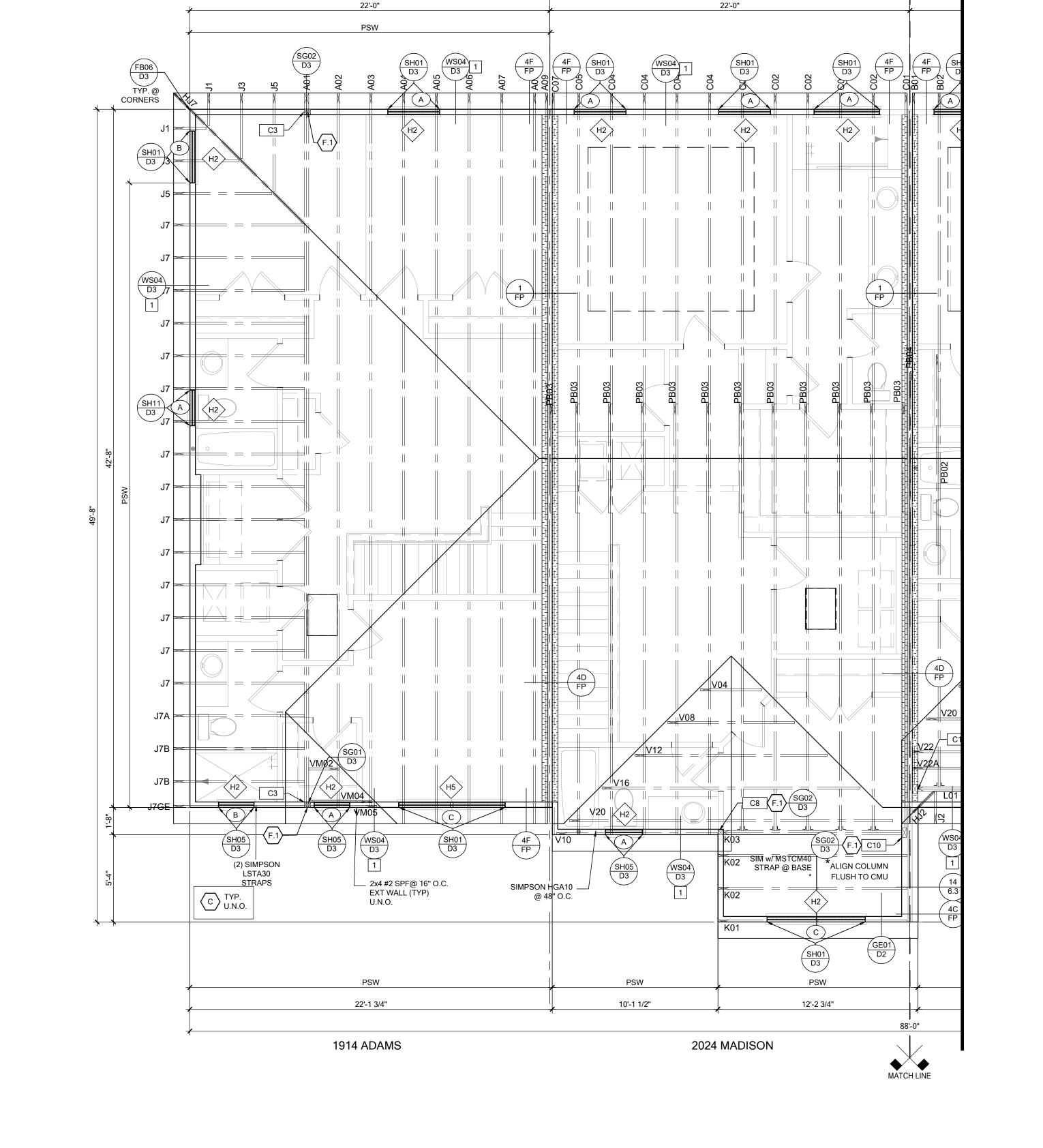
ROOF SHEATHING:

<u>TILE:</u> $^{15}/_{32}$ " EXP. 1 ($^{32}/_{16}$)

RSRS-04 (3" x .120") NAILS

DESIGN DESCRIPTION





ENGINEERED ROOF PER ASCE 7-16 ROOF DESIGN ALLOWABLE

COMPONENTS AND CLADDING WIND PRESSURES AND SUCTIONS

FOR MEAN ROOF HEIGHT ≤ 25 ft

(+) VALUE DENOTES PRESSURE

(-) VALUE DENOTES SUCTION

ROOF 1 2e 2n 2r 3 3e 3r

ASTM F1667 RSRS-01 (8d) NAILS @ 6" O.C. ON EDGE AND 6" O.C IN FIELD

HIP -43.90 -60.59 -60.59 -60.59

. PER CODE ASTM F1667 RSRS-01 REFERENCE TO 8d (2 $\frac{3}{8}$ " x 0.113") NAILS

ZONE 2e, 2n, 2r: ASTM F1667 RSRS-01 (8d) NAILS @ 4" O.C. ON EDGE AND 4" O.C IN FIELD

ZONE 3, 3e, 3r: ASTM F1667 RSRS-01 (8d) NAILS @ 4" O.C. ON EDGE AND 4" O.C IN FIELD

WHERE THE SHEATHING THICKNESS IS GREATER THAN 15/32", SHEATHING SHALL BE

ATTACH 2x4 #2 SYP BLOCKING @ 16" O.C FIRST 4 BAYS WITH (2) 12d NAILS EA. END. ATTACH ROOF SHEATHING TO RAFTERS W/ BLOCKING PER NAILING SCHEDULE.

HIP ROOF >20 TO 27 DEG.

FASTENED WITH ASTM F1667 RSRS-03 10d (2½" x 0.131") NAILS OR ASTM F1667

GABLES- DROP GABLE END & (1) ADDITIONAL DROPPED TRUSS 2x4 #2 SYP

OUTLOOKER RAFTER W/ BLOCKING @ 16" O.C. IF NO DROPPED GABLE END,

GABLE ROOF > 20 TO 27 DEG.
[4:12]-[6:12]

GABLE -43.86 -43.86 -69.97 -69.97

ROOF NAILING SCHEDULE/ NAILING ZONES (SHINGLE AND TILE):

WIND SPEED (ULTIMATE) 150.0 MPH WIND SPEED (ALLOWABLE) 116.2 MPH

SHINGLE: $\frac{7}{16}$ " EXP. 1 ($\frac{24}{16}$) or $\frac{15}{32}$ " EXP. 1 ($\frac{32}{16}$)

EXPOSURE CÀTEGORY

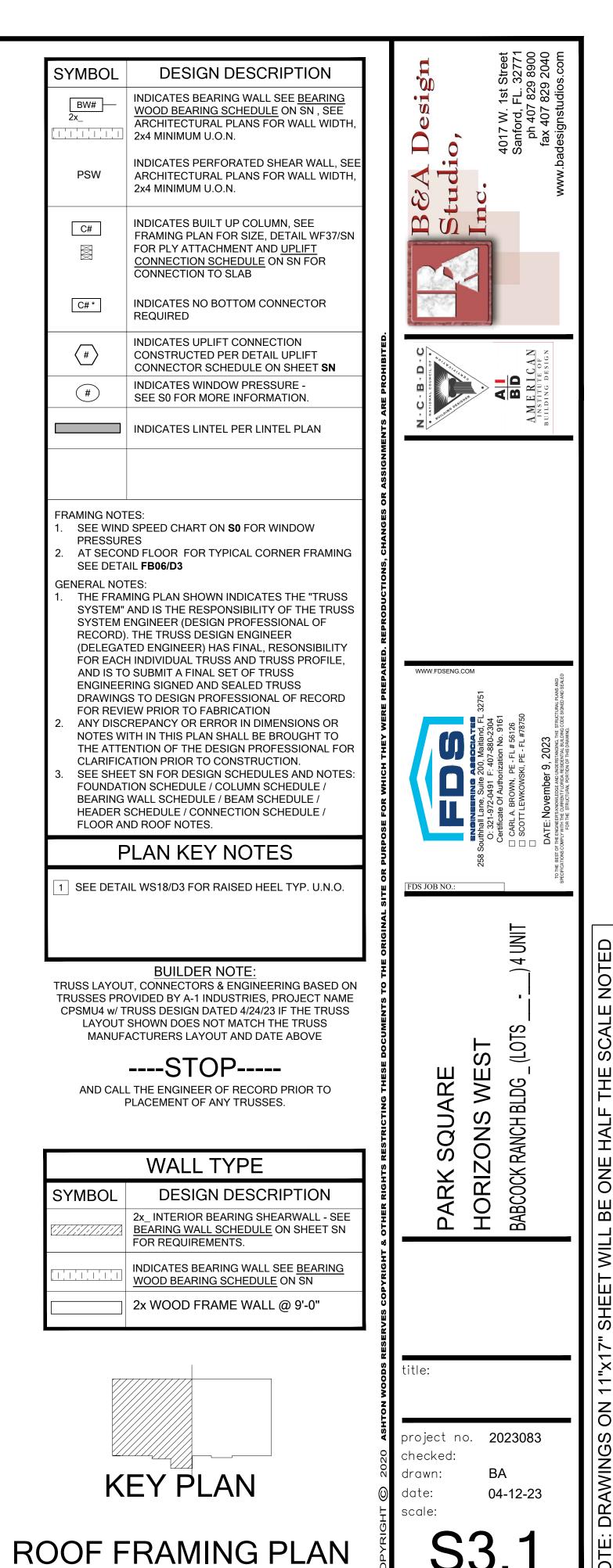
ROOF SHEATHING:

TILE: $^{15}/_{32}$ " EXP. 1 ($^{32}/_{16}$) NOTE:

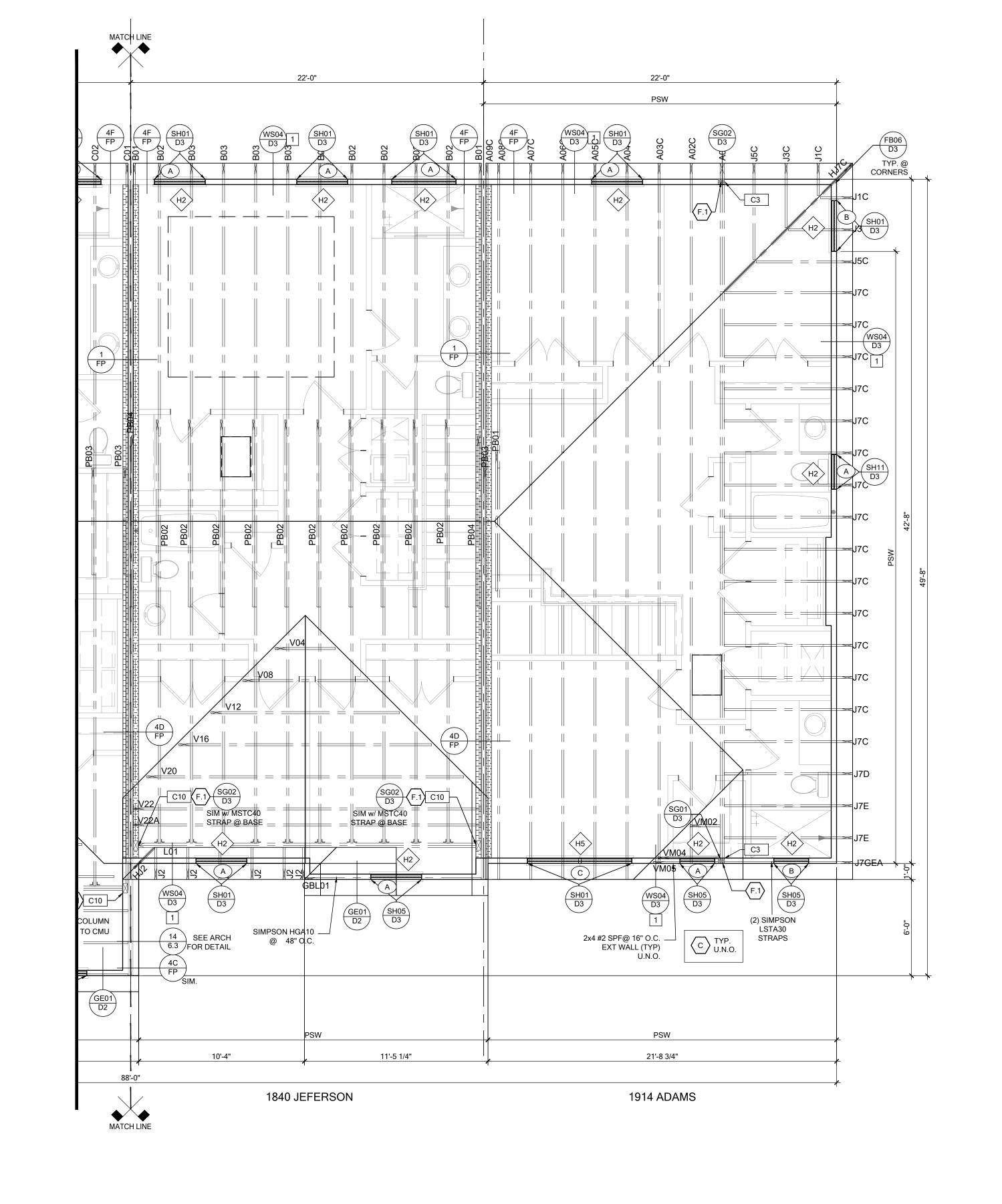
RSRS-04 (3" x .120") NAILS

WIND AREA

(SQ FEET) AREA MATCH LINE



SCALE: 1/4" = 1'-0"



ENGINEERED ROOF PER ASCE 7-16 ROOF DESIGN ALLOWABLE

COMPONENTS AND CLADDING WIND PRESSURES AND SUCTIONS

FOR MEAN ROOF HEIGHT ≤ 25 ft

(+) VALUE DENOTES PRESSURE
(-) VALUE DENOTES SUCTION

ROOF 1 2e 2n 2r 3 3e 3r

ASTM F1667 RSRS-01 (8d) NAILS @ 6" O.C. ON EDGE AND 6" O.C IN FIELD

HIP -43.90 -60.59 -60.59 -60.59

I. PER CODE ASTM F1667 RSRS-01 REFERENCE TO 8d (2 $\frac{3}{8}$ " x 0.113") NAILS

GABLES- DROP GABLE END & (1) ADDITIONAL DROPPED TRUSS 2x4 #2 SYP

OUTLOOKER RAFTER W/ BLOCKING @ 16" O.C. IF NO DROPPED GABLE END, ATTACH 2x4 #2 SYP BLOCKING @ 16" O.C FIRST 4 BAYS WITH (2) 12d NAILS EA. END. ATTACH ROOF SHEATHING TO RAFTERS W/ BLOCKING PER NAILING SCHEDULE.

HIP ROOF >20 TO 27 DEG.

GABLE ROOF > 20 TO 27 DEG. [4:12]-[6:12]

ZONE 2e, 2n, 2r: ASTM F1667 RSRS-01 (8d) NAILS @ 4" O.C. ON EDGE AND 4" O.C IN FIELD

ZONE 3, 3e, 3r: ASTM F1667 RSRS-01 (8d) NAILS @ 4" O.C. ON EDGE AND 4" O.C IN FIELD

WHERE THE SHEATHING THICKNESS IS GREATER THAN $^{15}/_{32}$ ", SHEATHING SHALL BE FASTENED WITH ASTM F1667 RSRS-03 10d (2½" x 0.131") NAILS OR ASTM F1667

GABLE -43.86 -43.86 -69.97 -69.97

ROOF NAILING SCHEDULE/ NAILING ZONES (SHINGLE AND TILE):

WIND SPEED (ALLOWABLE) 116.2 MPH

SHINGLE: $\frac{1}{16}$ " EXP. 1 ($\frac{24}{16}$) or $\frac{15}{32}$ " EXP. 1 ($\frac{32}{16}$)

EXPOSURE CATEGORY

ROOF SHEATHING:

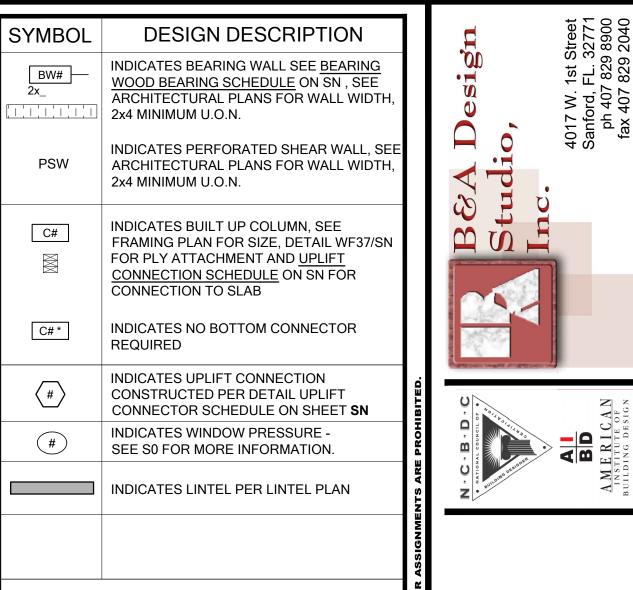
TILE: 15/32" EXP. 1 (32/16)

RSRS-04 (3" x .120") NAILS

WIND AREA

(SQ FEET)

AREA



FRAMING NOTES:

SEE WIND SPEED CHART ON **S0** FOR WINDOW PRESSURES

2. AT SECOND FLOOR FOR TYPICAL CORNER FRAMING SEE DETAIL **FB06/D3**

GENERAL NOTES:

THE FRAMING PLAN SHOWN INDICATES THE "TRUSS SYSTEM" AND IS THE RESPONSIBILITY OF THE TRUSS SYSTEM ENGINEER (DESIGN PROFESSIONAL OF RECORD). THE TRUSS DESIGN ENGINEER (DELEGATED ENGINEER) HAS FINAL, RESONSIBILITY FOR EACH INDIVIDUAL TRUSS AND TRUSS PROFILE, AND IS TO SUBMIT A FINAL SET OF TRUSS ENGINEERING SIGNED AND SEALED TRUSS DRAWINGS TO DESIGN PROFESSIONAL OF RECORD

FOR REVIEW PRIOR TO FABRICATION

2. ANY DISCREPANCY OR ERROR IN DIMENSIONS OR NOTES WITH IN THIS PLAN SHALL BE BROUGHT TO THE ATTENTION OF THE DESIGN PROFESSIONAL FOR CLARIFICATION PRIOR TO CONSTRUCTION.

3. SEE SHEET SN FOR DESIGN SCHEDULES AND NOTES: FOUNDATION SCHEDULE / COLUMN SCHEDULE / BEARING WALL SCHEDULE / BEAM SCHEDULE / HEADER SCHEDULE / CONNECTION SCHEDULE / FLOOR AND ROOF NOTES.

PLAN KEY NOTES

SEE DETAIL WS18/D3 FOR RAISED HEEL TYP. U.N.O.

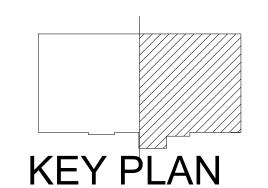
BUILDER NOTE:

TRUSS LAYOUT, CONNECTORS & ENGINEERING BASED ON TRUSSES PROVIDED BY A-1 INDUSTRIES, PROJECT NAME CPSMU4 w/ TRUSS DESIGN DATED 4/24/23 IF THE TRUSS LAYOUT SHOWN DOES NOT MATCH THE TRUSS MANUFACTURERS LAYOUT AND DATE ABOVE

----STOP-----

AND CALL THE ENGINEER OF RECORD PRIOR TO PLACEMENT OF ANY TRUSSES.

WALL TYPE							
SYMBOL	DESIGN DESCRIPTION						
	2x_ INTERIOR BEARING SHEARWALL - SEE BEARING WALL SCHEDULE ON SHEET SN FOR REQUIREMENTS.						
	INDICATES BEARING WALL SEE <u>BEARING</u> WOOD BEARING SCHEDULE ON SN						
	2x WOOD FRAME WALL @ 9'-0"						



ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0"

project no. **2023083**

04-12-23

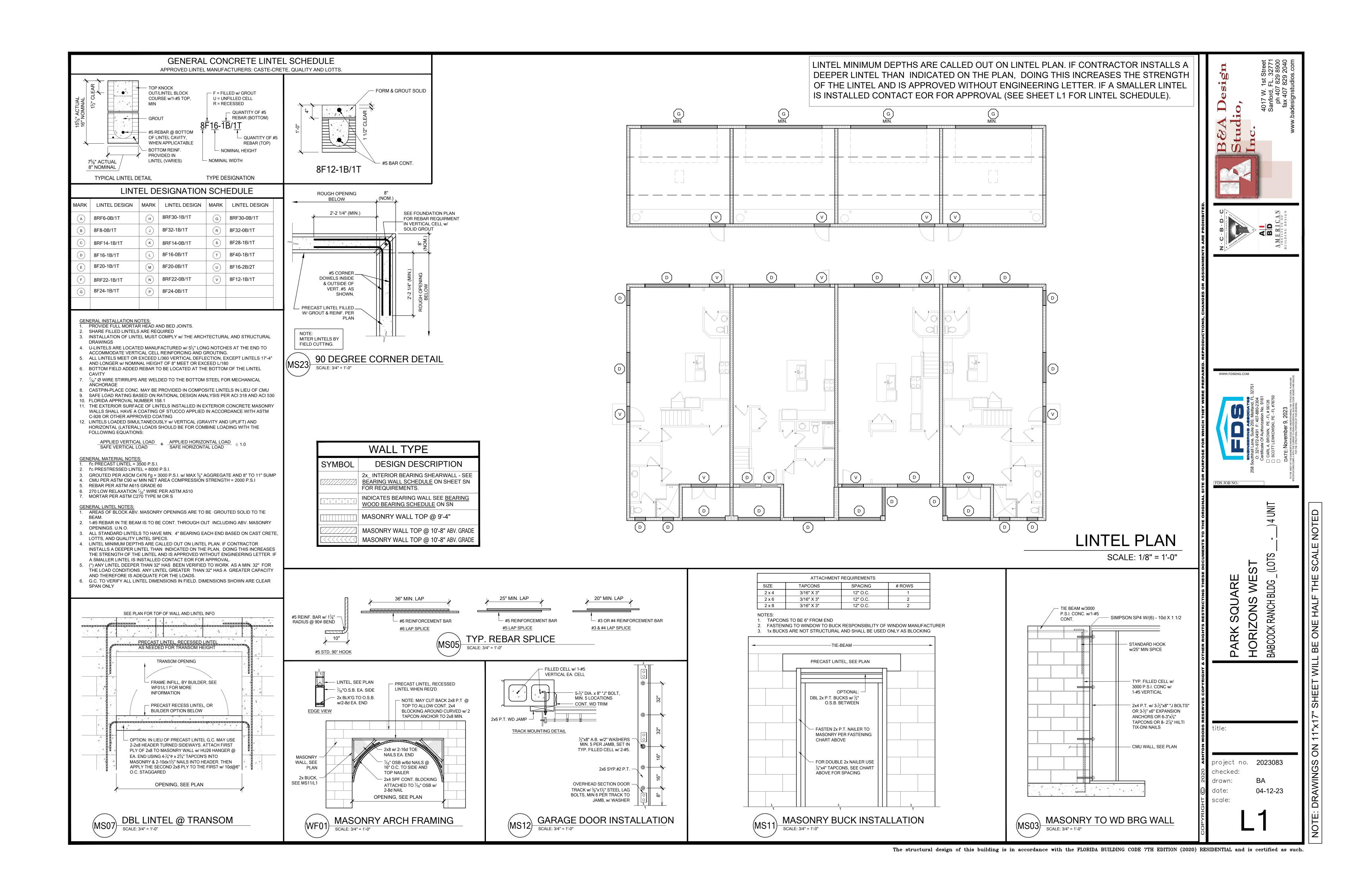
checked:

drawn:

date: scale:

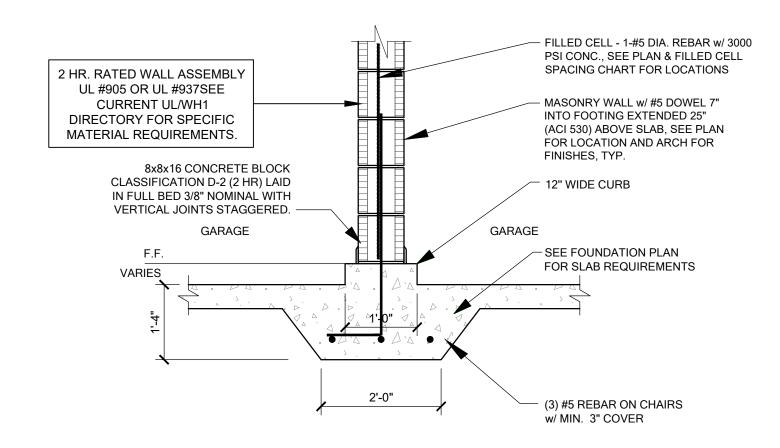
FDS JOB NO.:

DRAWINGS ON 11"x17" SHEET WILL BE ONE HALF TH

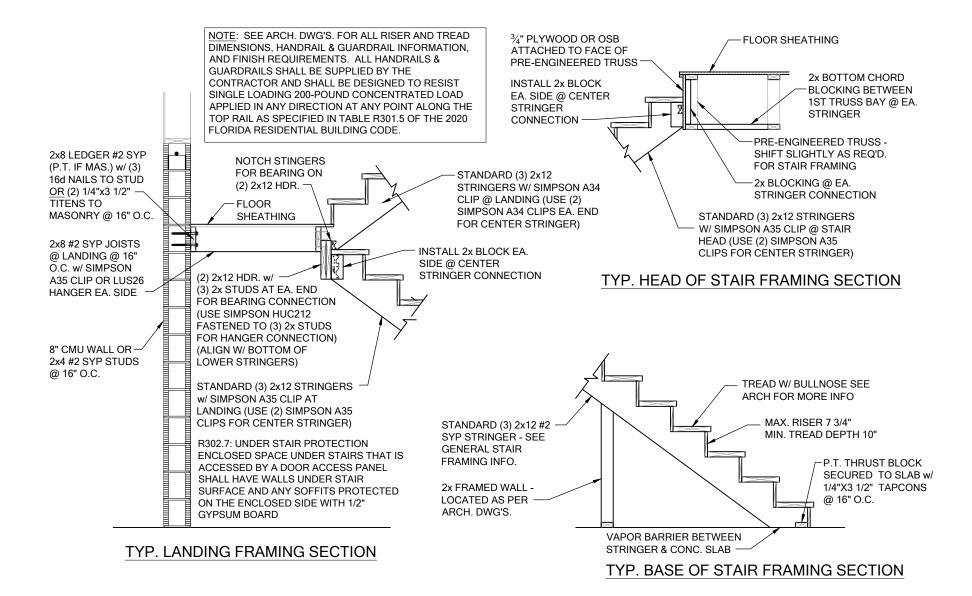


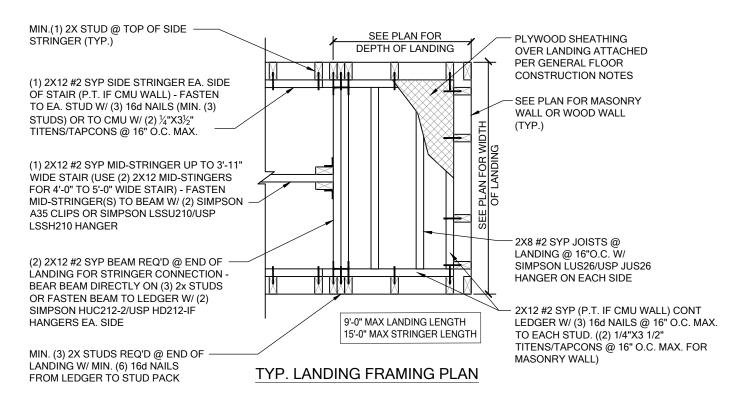
PARTY WALL FOOTING

SCALE: 3/4" = 1'-0"



GARAGE PARTY WALL FOOTING





CAST CRETE OR QUALITY/ LOTTS LINTEL LOAD SPECIFICATIONS

OR QUALITY		SAFE LOAD - POUNDS PER LINEAR FOOT										
	TYPE	0110	8F8-0B	8F12-0B	8F16-0B	8F20-0B	8F24-0B	8F28-0B	8F32-0E			
LENG.T.H		8U8	8F8-1B	8F12-1B	8F16-1B	8F20-1B	8F24-1B	8F28-1B	8F32-1E			
0' 10" (71") DD) FOACT	0071	3069	4605	6113	7547	8974	10394	11809			
2'-10" (34") PR	(ECAST	2231	3069	4605	6113	7547	8974	10394	11809			
7' C" (40") DE) F C A C T	0071	3069	3719	5163	6607	8054	9502	10951			
3'-6" (42") PR	(ECAST	2231	3069	4605	6113	7547	8974	10394	11809			
4'-0" (48") PR	PECAST	1966	2561	2751	3820	4890	5961	7034	8107			
- 0 (+0) 110	(LUAJ)	1900	2693	4605	6113	7547	8974	10394	11809			
4'-6" (54") PR	PECAST.	1599	1969	2110	2931	3753	4576	5400	6224			
	(20/101	1333	2189	4375	6113	7547 ₍₇₎		10294	11809			
5'-4" (64") PR	PECAST	1217	1349	1438	1999	2560	3123	3686	4249			
J + (U+) IN	(LUAJ)	1217	1663	3090	5365	7547 (36)	1. /					
5'-10" (70") PR	PECAST	1062	1105	1173	1631	2090	2549	3009	3470			
	(20/101	1002	1451	2622	4360	7168 (45)						
6'-6" (78") PR	RECAST	908	1238	2177	3480	3031	3707	4383	5061			
		300	1238	2177	3480	5381	8360	10394(37)				
7'-6" (90") PR	PECAST.	743	1011	1729	2632	2205	2698	3191	3685			
		743	1011	1729	2661	3898	5681	8467 (44)				
9'-4" (112") PR	RECAST	554	699	1160	1625	2564	3486	2818	3302			
		334	752	1245	1843	2564	3486	4705 (37)				
10'-6" (126") PR	RECAST	475	535	890	1247	2093	2777	2163	2536			
		170	643	1052	1533	2093	2781	3643 (38)				
11'-4" (136") PR	RECAST	362	582	945	1366	1846	2423	3127	4006			
			582	945	1366	1846	2423	3127	4006			
12'-0" (144") PR	RECAST	337	540	873	1254	1684	2193	2805	3552			
			540 471	873 755	1254 1075	1684	2193	2805	3552 2883			
13'-4" (160") PR	RECAST	296				1428	1838	2316				
. ,			471 424	755 706	1075 1002	1428 1326	1838 1697	2316 2127	2883 2630			
14'-0" (168") PR	RECAST	279	442	706	1002	1326	1697	2127	2630			
			NR	NR	NR	NR	NR	NR	NR			
14'-8" (176") PR	RESTRESSED	N.R.	458	783	1370	1902	2245	2517	2712			
			NR	NR	NR	NR	NR	NR	NR			
15'-4" (184") PR	KEZIKEZZED	N.R.	412	710	1250	1733	2058	2320	2513			
47' 4" (000") 55			NR	NR	NR	NR	NR	NR	NR			
17'-4" (208") PR	KEZIKEZZED	N.R.	300	548	950	1326	1609	1849	2047			
10' 4" (070") DD			NR	NR	NR	NR	NR	NR	NR			
19'-4" (232") PR	KESIKESSED	N.R.	235	420	750	1037	1282	1515	1716			
21'-4" (256") PR	PECTDECCEN	ND	NR	NR	NR	NR	NR	NR	NR			
21 -4 (230) PR	resinessed	N.R.	180	340	598	845	1114	1359	1468			
22'-0" (264") PR	PECTRECCEN	NO	NR	NR	NR	NR	NR	NR	NR			
ZZ -U (ZU4) PR	LJII\LJJEU	N.R.	165	315	550	784	1047	1285	1399			
24'-0" (288") PR	DDCCTDCCCC		NR	NR	NR	NR	NR	NR	NR			
Z4 -U (Z00) PR	(L) INEQUED	N.R.	129	250	450	654	884	1092	1222			

(#) THE NUMBERS IN PARENTHESIS ARE PERCENT REDUCTIONS FOR GR40 FIELD ADDED REBAR.

SAFE UPLIFT LOADS FOR 8" PRECAST & PRESTRESSED U-LINTELS

•	CACTO ITY/LOTTS	SAFE LOAD - POUNDS PER LINEAR FOOT									
	TYPE	8F8-1T	8F12-1T	8F16-1T	8F20-1T	8F24-1T	8F28-1T	8F32-1T			
LENG.T.H		8F8-2T	8F12-2T	8F16-2T	8F20-2T	8F24-2T	8F28-2T	8F32-2T			
0' 40" (74")	DDEOACT	1972	3173	4460	5747	7034	8321	9608			
2'-10" (34")	PRECAST	1972	3173	4460	5747	7034	8321	9608			
7' 0" (40")	DDEOACT	1569	2524	3547	4569	5591	6613	7636			
3'-6" (42")	PRECASI	1569	2524	3547	4569	5591	6613	7636			
4'-0" (48")	DDECAST	1363	2192	3079	3966	4853	5740	6627			
4 -0 (40)	FILCASI	1363	2192	3079	3966	4853	5740	6627			
4'-6" (54")	PRECAST	1207	1940	2724	3508	4292	5077	5861			
+ 0 (3+)	TILCAST	1207	1940	2724	3508	4292	5077	5861			
E' 4" (64")	DDFCACT	1016	1632	2290	2949	3607	4265	4924			
5'-4" (64")	PRECASI	1016	1632	2290	2949	3607	4265	4924			
5'-10" (70")	DDECAST	909	1492	2093	2694	3295	3897	4498			
3 - 10 (70)	FILCASI	929	1492	2093	2694	3295	3897	4498			
6'-6" (78")	DRECAST	835 (12)	1340	1880	2419	2959	3498	4038			
0 0 (70)	TILCAST	835	1340	1880	2419	2959	3498	4038			
7' 6" (00")	0") PRECAST	727 ₍₂₃₎	1021	1634 (12)	2102 (11)	2571 ₍₁₀₎	3039 (10)	3508 (
7 -6 (90)		727	1166	1634	2102	2571	3039	3508			
0'-4" (112")	') PRECAST	591	680	1133 (15)	1471 (15)	1811(15)	2152(16)	2494 (1			
J + (112)		591	851	1326	1705	2084	2463	2842			
10' 6" (126")	') PRECAST	530	552	914 (15)	1185 (15)	1458 (15)	1732 (15)	2007 (1			
10 -0 (120)		530	686	1183	1526	1865	2204	2544			
11' 1" (176")) PRECAST	474	485	798 (15)	1034 (15)	1272 (15)	1510 (15)	1749 ₍₁			
11 -4 (130)		494	599	1028	1422	1738	2053	2369			
12'-0" (144")	DDECAST	470 (9)	441	723 (14)	936 (14)	1151(15)	1366 (15)	1582 (1			
12 -0 (144)	PRECASI	470	543	928	1349	1649	1948	2247			
13'-4" (160")	DDFCACT	418 (15)	373	606 (14)	783 (14)	962 (14)	1141 ₍₁₄₎	1321 ₍₁			
13 -4 (100)	PRECASI	428	455	770	1145	1444	1718	1993			
14' 0"(160")	DDECACT	384 ₍₁₅₎	346	559 (14)	723 (14)	887 (14)	1052 (14)	1218 (1			
14'-0" (168")	PRECASI	410	420	709	1050	1434 (8)					
14' 0" (176")	PRESTRESSED	239	323	519 (13)	671 (13)	823 (13)	976 (14)	1129 (1			
14 -0 (1/0)		246	390	655	968	1324 (8)	1625 (11)	1874 (1			
15' 4" (104")	PRESTRESSED	224	302	485 (13)	626 (13)	767 (13)	909 (13)	1052 (1			
13 -4 (164)		230	364	609	897	1224 (8)	1562 (14)	1801 (1			
17'-4" (208")	DDECTDECCED	187	255	404 (12)	520 (12)	637 (12)	754 (12)	872 (1			
17 -4 (200)	FIVESTIVESSED	192	303	500	732	993 (8)	1268 (14)	1470 (1			
19'-4" (232")	PRESTRESSEN	162	222	347 (11)	446 (11)	546 (12)	646 (12)	746 (1			
19 -4 (202)	LUTCOLKEODEN	166	261	424	616	831 (8)	1057 (14)	1225 (1			
21'-4" (256")	PRESTRESSEN	142	198	306 (11)	393 (11)	480 (11)	567 (11)	654 (1			
	PRESTRESSED -	142	230	369	531	713 (7)	903 (13)	1046 (1			
22'-0" (264")	PRESTRESSEN	137	192	295 (10)	378 (11)	461 (10)					
	I INCOTINCOOLD	137	221	354	508	681 ₍₇₎					
24' 0"(200")	DDECTDECCED	124	175	267 (10)	341 (10)						
24'-0" (288")	LVEDIVEDOED	124	200	316	450	600 (7)	756 (12)	875 (1			

(#) THE NUMBERS IN PARENTHESIS ARE PERCENT REDUCTIONS FOR GR40 FIELD ADDED REBAR.

project no. **2023083** 04-12-23

FDS JOB NO.:

checked: drawn:

date:

